# INVESTIGATION OF CEMENT BASED ADHESIVES TO REPLACE EPOXY BOND IN CFRP/CONCRETE COMPOSITE

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Dissertation submitted in partial fulfillment of the requirements for the Degree Master of Engineering in Structural Engineering Design

Department of Civil Engineering

University of Moratuwa Sri Lanka

March 2016



# **DECLARATION**

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Dr.(Mrs), J.C.P.H.O	Gamage					

Supervisor

## **ABSTRACT**

Fibre Reinforced Polymer (FRP) application is a very effective way to repair and strengthen structures that has become structurally weak over their life span. The strengthening and rehabilitation of existing structures are major issues worldwide. In most situations, strengthening is required when there is an increase in the applied load, human error in the initial construction, a legal requirement to comply with updated versions of existing codes, or as a result of the loss of strength due to deterioration over time. Fibre-Reinforced Polymer (FRP) strengthening systems are enjoying a great deal of popularity as a result of the unique properties of FRPs - namely, their light weight, fatigue resistance non-corrosive characteristics and ease of application. The repair and strengthening technique with epoxybonded advanced composites has been applied to a large number of bridges around the world. At elevated temperatures, normally beyond the glass transition temperatures of epoxy adhesive, the mechanical properties of the polymer matrix deteriorate rapidly. It will be very beneficial if they can be replaced by cement grout bonding agents such as modified concrete, in order to produce fire-resistant strengthening systems.

This report includes the investigation of the flexural behavior of FRP-strengthened reinforced concrete beams using epoxy adhesive and cement grout adhesive. A total of ten RC beams were cast. All of them were having the same cross section of 100 mm X 150 mm and the span of 500 mm. Two beams were tested as control beams and another two beams were strengthened with CFRP using epoxy adhesive. Remaining six beams were strengthened with CFRP using cement grout with different bonding arrangements.

Finally, experimental results were compared with theoretical results and previous research data. The results showed the Compared with theoretical results and previous research data. The results showed the Compared with the results are showed that the same were strengthened using capacity as going a going described when beams were ability to increase the ultimate load carrying capacity as well. Furthermore, use of anchoring system to strengthen beams is an effective technique.

**KEYWORDS:** Carbon Fiber Reinforced Polymers (CFRP), Reinforced Concrete Beams, Failure Load, Anchoring system, Epoxy, Cement grout

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University of Moratuwa, Sri Lanka.

Mr Nama Jana Edeotronian Type Scene and blast the test speciments.

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# TABLE OF CONTENT

DECLARATION	i
ABSTRACT	ii
ACKNOWLEDGEMENT	iii
TABLE OF CONTENT	iv
LIST OF FIGURES	vii
LIST OF TABLES	ix
LIST OF ABBREVIATIONS	x
LIST OF SYMBOLS	xi
LIST OF APPENDICES	xii
CHAPTER 01: INTRODUCTION	1
1.1 General	1
1.2 Objectives of the research study	2
1.3 Methodology	3
1.4 Arrangement of the thesis report	5
1.5 Conclusions	
CHAPTER D. LITERNIVERSETREVIEW OF ATUWA, Sri Lanka	
2.1 General for Education of Pheses & Dissertations	
2.2 Material Characteristics.mrt.ac.lk	8
2.2.1 Fiber Reinforced Polymer	
2.2.2 Adhesives	
2.3 FRP/Concrete composite	
2.4 FRP strengthening techniques	
2.4.1 Wet layup systems	14
2.4.2 Prepreg systems	15
2.4.3 Pre-cured systems	15
2.4.4 Near-surface-mounted systems	15
2.5 Failure modes in flexural strengthened RC beams	16
2.6 Temperature and humidity considerations	18
2.7 Thermal properties of materials	19
2.7.1 CFRP	19
2.7.2 Epoxy	20

2.7.4 CFRP/epoxy/cement complex	22
2.8 Comparison between epoxy and cement based adhesive	22
CHAPTER 03: EXPERIMENTAL STUDY	34
3.1 Experimental program	34
3.2 Material properties	35
3.2.1 Concrete	36
3.2.2 Reinforcement	38
3.2.3 CFRP specification	38
3.2.4 Primer specification	39
3.3 Specimen preparation	41
3.3.1 Reinforcement cage	41
3.3.2 Casting of beams	42
3.3.3 Surface preparation	42
3.3.5 Application of CFRP	45
3.4 Testing of materials and specimens	49
3.4.1 Materials testing	49
3.4.2 Specimens details	52
3.4.3 Specimens testing sity of Moratuwa, Sri Lanka.	53
3.5 Expendental results onic Theses & Dissertations	
3.6 Analysis of resultsw.lib.mrt.ac.lk	56
3.6.1 Specimens C1 and C2	56
3.6.2 Specimens A-E1 and A-E2	57
3.6.3 Specimens B-C1 and B-C2	57
3.6.4 Specimens C-PC1 and C-PC2	58
3.6.5 Specimens D-PC1 and D-PC2	59
3.7 Load vs. deflection behavior in flexural strengthened beams	59
3.8 Conclusion	62
CHAPTER 04: THEORITICAL ANALYSIS	64
4.1 Introduction	64
4.2 Theoretical calculations for load carrying capacity	64
4.2.1 Prediction of Flexural/ Shear capacities for control beams	64
4.3 Comparison of theoretical results and experimental results	80
4.4 Results and discussion	83
4.5 Conclusion	84
CHAPTER 05: COMPARISON WITH PREVIOUS STUDIES	85

5.1 Comparison of strength gains with previous researchers	85
5.2 Comparison of failure modes with previous researchers	85
5.3 Conclusion	86
CHAPTER 06: CONCLUSIONS AND RECOMMENDATIONS	87
5.1 Conclusions and Recommendations	87
5.2 Further studies	88
REFERENCES	90
Appendix A: Details of flexural capacity enhancement of beams	94
Appendix B: Details of flexural capacity enhancement of beams using Cement based adhesive	1 .
Appendix C: Details of Cement adhesive mix ratios	103
Appendix D: Details of testing data	108



# LIST OF FIGURES

Figure 1.1: Methodology Flow Chart.	4
Figure 2.1: Various forms of FRP Materials	7
Figure 2.2: Fiber directions in composite materials	7
Figure 2.3: Stress-strain behavior of FRP compared to steel	8
Figure 2.4: Internal strain and stress distribution for a rectangular section under flexure at ultimate stage	14
Figure 2.5: Wet layup system.	15
Figure 2.6 : NSM system	16
Figure 2.7: Failure modes of FRP-strengthened RC beams	17
Figure 2.8 : Temperature Vs failure load (Wu et.al 2005)	21
Figure 2.9 : Temperature failure load relation of double lap shear test (Tadeu and Branco 2000)	21
Figure 2.10: Different forms of CFRP wrapped	30
Figure 2.11: Different forms of CFRP wrapping on prisms	31
Figure 2.12: Effect of CFRP wrapping on flexural strength	32
Figure 3.1. Reinforcements of page 1.1. Reinforcements of page 2.1. Reinforcements of page 3.1. Reinforcements of	38
www.lib.mrt.ac.lk  Figure 3.3: Reinforcement arrangement for flexural strengthened beams	41
Figure 3.4: Casting of RC beam samples	42
Figure 3.5: Sand blasting of beams	43
Figure 3.6: After surface preparation of beam	43
Figure 3.7: Cleaning surface using wire brush	43
Figure 3.8: Two part Primer	44
Figure 3.9: Application of Primer	44
Figure 3.10: Primed surface of beam	45
Figure 3.11: CFRP sample preparing	45
Figure 3.12: Saturant sample A &B.	45
Figure 3.13: Application of saturant	46
Figure 3.14: Attaching of CFRP and air removing	46
Figure 3.15: Application of saturant on pasted CFRP laminate	46
Figure 3.16: CFRP laminate pasted with cement grout	47

Figure 3.17: Applying of cement grout after pasted CFRP laminate	4
Figure 3.18: Pasted CFRP laminate on primer coated beam sample	4
Figure 3.19: Sketch of "D" type beam sample	4
Figure 3.20: CFRP laminate on primer coated beam sample and strengthen with 'U' Wtrap	4
Figure 3.21: Concrete Cube testing	4
Figure 3.22: Cement grout testing	5
Figure 3.23: Load Vs Elongation for steel specimen no 01	5
Figure 3.24: Load Vs No of revolutions for steel specimen no 02	5
Figure 3.25: Schematic diagram for the test setup.	5
Figure 3.26: Testing of samples	5
Figure 3.27: Failure pattern of C1	5
Figure 3.28: Failure pattern of C2	5
Figure 3.29: Failure pattern of A-E1	5
Figure 3.30: Failure pattern of A-E2	5
Figure 3.31: Failure pattern of B-C1 University of Moratuwa, Sri Lanka.  Figure 3.32 Failure pattern of B-C2 Electronic Theses & Dissertations  Figure 3.33 Failure pattern of B-C1  Figure 3.34 Failure pattern of B-C1	5 5 5
Figure 3.34: Failure pattern of C-PC2	5
Figure 3.35: Failure pattern of D-PC1	5
Figure 3.36: Failure pattern of D-PC2	5
Figure 3.37a: Load Vs mid span deflection (used CFRPsamplle1,epoxy adhesive)	6
Figure 3.37b: Load Vs mid span deflection (used CFRPsamplle2, cement grout adhesive)	6
Figure 3.38: Serviceability failure load Vs beam sample	6
Figure 4.1: Schematic diagram of Non strengthened test beam	6
Figure 4.2: Stress distribution in beam	6
Figure 4.3: Internal strain and stress distribution for a rectangular section under flexure at ultimate stage.	7
Figure 4.4: Schematic diagram of strengthened test beam	7
Figure 4.5: Comparison of Theoretical results and Experimental results	8
Figure 5.1: Results of the current study and previous studies of strength gain	8

# LIST OF TABLES

Table 2.1: Mechanical Properties of Fibers (Fib-Bulletin et al, 2001)	9
Table 2.2: Comparison of Fiber materials with Steel (Fib-Bulletin et al, 2001)	9
Table 2.3: Mechanical properties of adhesives (Anders et al, 2003)	11
Table 2.4: Mechanical properties of polymer matrix materials (Morgan et al.2008)	20
Table 2.5: Test results (Adhikarinayake S.R et.al.2013)	24
Table 2.6: Experimental results (Hashemi & Al-Mahaidi, 2012)	26
Table 2.7: Test results (Hashemi & Al-Mahaidi, 2008)	27
Table 2.8: Effect of CFRP on flexural strength (Shehab El – Din & Mohamed, 2013)	32
Table 3.1: Details of test beams.	34
Table 3.2: Density of concrete constituents	36
Table 3.3: Volume of concrete yield per bag of cement	37
Table 3.4: Required quantities of material per beam	37
Table 3.5: Properties of reinforcement materials	38
Table 3.6: CFRP manufactus specifications (BS AF: MBrade fabric, May 2009)	39
Table 3. The Friedrania factores specialisated about MBrace Specifi	40
Table 3.8. Manufacturer's specifications for saturant (BSAF, MBrace speci	41
Table 3.9: Cube testing data - cubes at the date of testing beams (Measured)	50
Table 3.10: Cube testing data	51
Table 3.11: Specimens details	53
Table 3.12: Ultimate failure loads for beams	55
Table 3.13: Failure modes of beams	56
Table 4.1: Design parameters for un-strengthened test beam	65
Table 4.2: Design parameters for strengthened test beam	72
Table 4.3: Analysis of Experimental results and theoretical results	82
Table 5.1: Results of the current study and previous studies of failure modes	86

## LIST OF ABBREVIATIONS

AFRP : Aramid Fibre Reinforced Polymer

ACI : American Concrete Institute

A<sub>s</sub> : Provided area of main tension reinforcement.

A<sub>sv</sub> : Provided area of shear reinforcement

b : Width of the section

CFRP : Carbon Fibre Reinforced Polymer

d : Depth of the section

E : Young's Modulus

 $F_c$ : Force in concrete compression zone.

 $f_{cu}$  : Characteristic strength of concrete.

FRP : Fibre Reinforced Polymer

 $F_t$ : Force in main tension Reinforcement.

f<sub>y</sub> : Characteristic tensile strength of main Reinforcement

f<sub>vv</sub> : Characteristic tensile strength of shear Reinforcement

GFRP
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IC bonding: WWW.lib.mrt.ac.lk
Intermediate crack bonding

IESL : Institution of Engineers of Sri Lanka

l : Length of specimen

M : Expected maximum moment.

NSM : Near Surface Mounted.

RC: Reinforced concrete

RF : Reinforcement

s : Spacing of shear links along the members

UOM : University of Moratuwa

w : Uniformly distributed self –weight of concrete element.

W : Applied point load.

x : Depth to the neutral axis

Z : lever arm

# LIST OF SYMBOLS

A<sub>f</sub> : nt<sub>f</sub>w<sub>f</sub>, area of FRP external reinforcement (mm<sup>2</sup>)

 $A_g$ : gross area of section (mm<sup>2</sup>)

 $A_s$ : area of nonprestressed steel reinforcement (mm<sup>2</sup>)

A<sub>st</sub> : total area of longitudinal reinforcement (mm<sup>2</sup>)

b : width of rectangular cross section (mm)

c : distance from extreme compression fiber to the neutral axis

(mm)

CE : environmental-reduction factor

D : distance from extreme compression fiber to the neutral axis

(mm)

d<sub>f</sub> : depth of FRP shear reinforcement (mm)

E<sub>c</sub> : modulus of elasticity of concrete (MPa)

E<sub>f</sub> : tensile modulus of elasticity of FRP (MPa)

E<sub>s</sub> : modulus of elasticity of steel (MPa)

f<sub>c</sub> University of Moratuwa, Sri Lanka.

f'c specified compressive strength of concrete (MPa)

 $f_{\rm f}$ : stress level in the FRP reinforcement(MPa)

 $f_{\rm f,s}$  : stress level in the FRP caused by a moment within the elastic

range of the member (MPa)

 $f_{\rm fe}$  : effective stress in the FRP; stress level attained at section

failure (MPa)

 $f^*_{fu}$ : ultimate tensile strength of the FRP material as reported by the

manufacturer (MPa)

 $f_{\rm fu}$  : design ultimate tensile strength of FRP (MPa)

F<sub>v</sub> : specified yield strength of nonprestressed steel reinforcement

(MPa)

h : overall thickness of a member (mm)

M<sub>n</sub> : nominal moment strength (N-mm)

p\*<sub>fu</sub> : ultimate tensile strength per unit width of the FRP

Reinforcement (N/mm)

 $T_g$ : glass-transition temperature, °F (°C)

b1 : ratio of the depth of the equivalent rectangular stress block to

the depth of the neutral axis

 $\in_{b}$  : strain level in the concrete substrate developed by a given

bending moment (tension in positive)

 $\in_{\text{bi}}$  : strain level in the concrete substrate at the time of the FRP

installation (tension is positive)

 $\in_{c}$  : stain level in the concrete

 $\epsilon_{cu}$  : maximum usable compressive strain of concrete

 $\in$ f : strain level in the FRP reinforcement

 $\in_{\text{fe}}$  : effective strain level in FRP reinforcement; strain level

attained at section failure

 $\in_{\text{fu}}$  : design rupture strain of FRP reinforcement

 $ext{$\in^*$}_{\text{fu}}$  : ultimate rupture strain of the FRP reinforcement

€s University of Moratuwa, Sri Lanka strain level in the nonprestessed steel reinforcemer

Electronic Theses & Dissertations strain corresponding to the yield strength of nonprestressed

steel reinforcement

φ : strength reduction factor

k<sub>a</sub> : efficiency factor for FRP reinforcement (based on the section

geometry)

k<sub>m</sub> : bond-dependent coefficient for flexure

y<sub>f</sub> : additional FRP strength-reduction factor

## LIST OF APPENDICES

Appendix A - Details of flexural capacity enhancement of beams

Appendix B - Details of flexural capacity enhancement of beams using Epoxy and

Cement based adhesive

Appendix C - Details of Cement adhesive mix ratios

Appendix D - Details of testing data